

Unbonded steel tube concrete

Autor(en): **Sato, Takanori / Tanaka, Nobuyuki / Orito, Yoshihiro**

Objektyp: **Article**

Zeitschrift: **IABSE reports = Rapports AIPC = IVBH Berichte**

Band (Jahr): **55 (1987)**

PDF erstellt am: **02.07.2024**

Persistenter Link: <https://doi.org/10.5169/seals-42816>

Nutzungsbedingungen

Die ETH-Bibliothek ist Anbieterin der digitalisierten Zeitschriften. Sie besitzt keine Urheberrechte an den Inhalten der Zeitschriften. Die Rechte liegen in der Regel bei den Herausgebern.

Die auf der Plattform e-periodica veröffentlichten Dokumente stehen für nicht-kommerzielle Zwecke in Lehre und Forschung sowie für die private Nutzung frei zur Verfügung. Einzelne Dateien oder Ausdrucke aus diesem Angebot können zusammen mit diesen Nutzungsbedingungen und den korrekten Herkunftsbezeichnungen weitergegeben werden.

Das Veröffentlichen von Bildern in Print- und Online-Publikationen ist nur mit vorheriger Genehmigung der Rechteinhaber erlaubt. Die systematische Speicherung von Teilen des elektronischen Angebots auf anderen Servern bedarf ebenfalls des schriftlichen Einverständnisses der Rechteinhaber.

Haftungsausschluss

Alle Angaben erfolgen ohne Gewähr für Vollständigkeit oder Richtigkeit. Es wird keine Haftung übernommen für Schäden durch die Verwendung von Informationen aus diesem Online-Angebot oder durch das Fehlen von Informationen. Dies gilt auch für Inhalte Dritter, die über dieses Angebot zugänglich sind.

Unbonded Steel Tube Concrete

Tubes en acier remplis de béton

Stahlrohre mit Betonfüllung ohne Verbund

Takanori SATO

Research Engineer

Res. Inst. of Shimizu Construction Co., Ltd.

Tokyo, Japan

Yoshihiro ORITO

Research Engineer

Nuclear Dep. of Shimizu Construction Co., Ltd.

Tokyo, Japan

Nobuyuki TANAKA

Research Engineer

Res. Inst. of Shimizu Construction Co., Ltd.

Tokyo, Japan

Yasushi WATANABE

Design Engineer

Design Dep. of Shimizu Construction Co., Ltd.

Tokyo, Japan

1. INTRODUCTION

The structure has been developed to apply to structural members subjected to large axial force such as the columns of highrise buildings, the columns of multistoried large span structures, the main poles of suspension bridges or the supports of underground structures. This is a new structural form, which is different in axial force supporting mechanism from conventional reinforced concrete (RC) or from steel reinforced concrete (SRC). The structure is constructed in such a way that a thin frictionless material (asphalt layer of 0.2mm thick) is applied to the inner surface of the cylindrical steel tube in which concrete is filled. The structure is named as "Unbonded Steel Tube Concrete" (UTC). This paper describes the principle and composition of the UTC and the outline of results of various tests.

2. PRINCIPLE OF UTC

In the UTC, axial force (N) is supported only by the filled concrete sections, and at the time axial stress $s_{\theta z}$ is generated in the steel tube as less as possible thanks to the frictionless material and only the restraining effect by circumferential stress $s_{\theta\theta}$ is expected. Bending moment (M) and shearing force (Q) are supported by both sections of the steel tube and filled concrete, similar to other structural forms. The comparison on the supporting mechanism of axial force (N) is shown in Figure-1. The stress conditions under combined force, when compared with that of bonded steel tube concrete (BTC) as a representative of other structural forms, are shown in Figure-2. On the assumption that the filled concrete is not failed as far as the steel tube is not failed, the design strength depends on the local yield or buckling of the steel tube. It is apparent from Figure-2 that the UTC has larger strength in bending moment (M) than the BTC, as axial force N becomes larger.

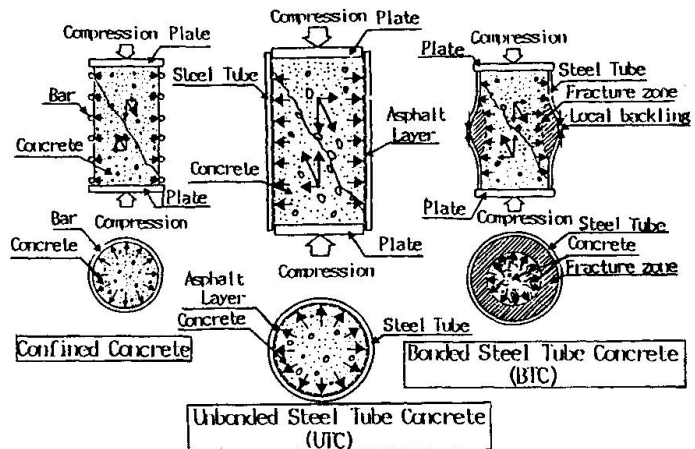


Fig.-1 Mechanism of UTC and Relative Forms

External Loads	$\begin{matrix} \uparrow N \\ \\ \downarrow N \end{matrix} + \begin{matrix} \curvearrowright M \\ \\ \curvearrowleft M \end{matrix} = \begin{matrix} \uparrow N \\ \\ \downarrow N \end{matrix} + \begin{matrix} \curvearrowright M \\ \\ \curvearrowleft M \end{matrix}$	Yielding Condition
UTC	$\begin{matrix} \text{Hoop Tension} \\ c_{\theta z} \ s_{\theta\theta} \end{matrix} + \begin{matrix} s_{\theta z} \\ c_{\theta z} \end{matrix} = \begin{matrix} s_{\theta z} \\ s_{\theta\theta} \end{matrix}$	$\sqrt{s_{\theta z}^2 + s_{\theta\theta}^2} - s_{\theta z} \cdot s_{\theta\theta} = f_y$ (Von-Mises Criterion) Gradually in Both Fibers
BTC	$\begin{matrix} \text{Only Concrete} \\ c_{\theta z} \end{matrix} + \begin{matrix} \text{Concrete and Steel} \\ c_{\theta z} \end{matrix} = \begin{matrix} s_{\theta z} \\ c_{\theta z} \end{matrix}$	$s_{\theta z} = f_y$ Concentratedly in Compression Fiber

Fig.-2 Stress Condition under Combined Force



3. BEAM-COLUMN JOINT

The 3 types shown in Figure-3 are considered as a joint of the UTC column with beam in building structures. Type A is expected to improve the strength of N, M, Q, type B and C improve only N.

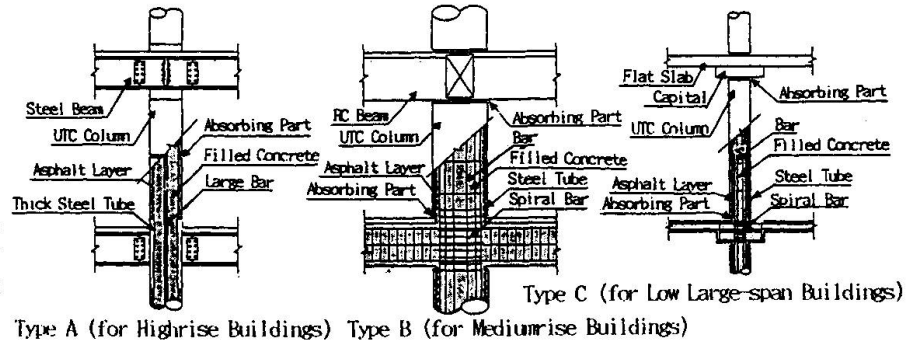


Fig.-3 Detail of Beam-Column Joint

4. OUTLINE OF RESULTS OF VARIOUS TESTS

4.1 Concentric compression test

A concentric compression test is carried out for 3 kinds of specimens U, B and R of which the UTC, the BTC and the positioning between the 2 are simulated respectively ($\phi 114 \times 600 \text{mm}$). In specimen R a frictionless material is not applied but force is loaded only to filled concrete section. As shown in Figure-4, specimen U has yielding strength N_y approximately 30% higher than specimen B, and having sufficient toughness. Specimen R is between them. The frictionless material effect is confirmed.

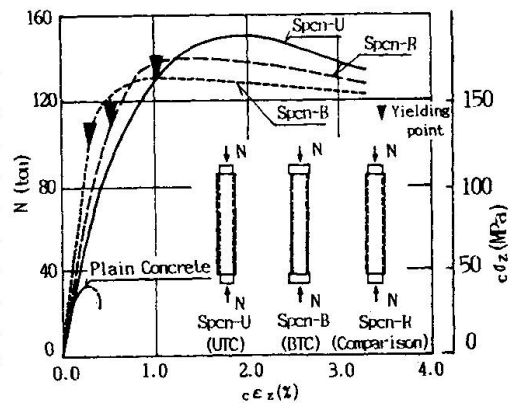


Fig.-4 Concentric Compression Test

4.2 Bending test

To compare the UTC and the BTC ($\phi 216 \times 1500 \text{mm}$), load is added to bend at constant axial force N . As shown in Figure-5, yielding bending strength P_y of the UTC is approximately twice that of the BTC. The difference depends on the intensity of constant axial force N . This is also understood from Figure-2. However, maximum bending strength P_u is almost equal, and has sufficient toughness.

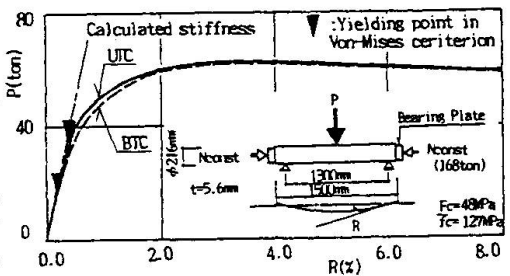


Fig.-5 Bending Test

4.3 Shear test

Even if the steel pipe and filled concrete are not bonded as in UTC, it is made clear experimentally ($\phi 216 \times 2800 \text{mm}$) and analytically that shear transfer occurs between them by side pressure distribution, as shown in Figure-6.

5. SUMMARY

"Unbonded Steel Tube Concrete" (UTC) which is different from other structural forms (RC and SRC) is proposed and in order to apply it to actual structures, various tests are carried out and described the outline of them.

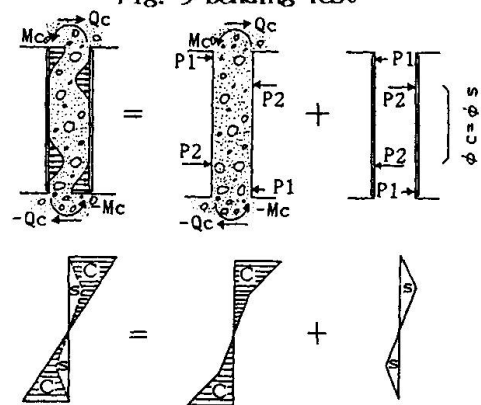


Fig.-6 Shear Transfer Mechanism

ACKNOWLEDGMENT

The authors would like to acknowledge their helps and advices of Professor Kazuo Suzuki of Osaka University and Professor Hiroyuki Aoyama of Tokyo University.

REFERENCES

(1) Y. Orito et al; "Study on Unbonded Steel Tube Concrete", Conference on Composite Construction by Engineering Foundation 1987.6 New Hampshire