

Zeitschrift: IABSE reports = Rapports AIPC = IVBH Berichte
Band: 69 (1993)

Artikel: Tuned mass dampers for continuous beams
Autor: Svensson, Eilif / Plum, Carsten Munk
DOI: <https://doi.org/10.5169/seals-52582>

Nutzungsbedingungen

Die ETH-Bibliothek ist die Anbieterin der digitalisierten Zeitschriften. Sie besitzt keine Urheberrechte an den Zeitschriften und ist nicht verantwortlich für deren Inhalte. Die Rechte liegen in der Regel bei den Herausgebern beziehungsweise den externen Rechteinhabern. [Siehe Rechtliche Hinweise.](#)

Conditions d'utilisation

L'ETH Library est le fournisseur des revues numérisées. Elle ne détient aucun droit d'auteur sur les revues et n'est pas responsable de leur contenu. En règle générale, les droits sont détenus par les éditeurs ou les détenteurs de droits externes. [Voir Informations légales.](#)

Terms of use

The ETH Library is the provider of the digitised journals. It does not own any copyrights to the journals and is not responsible for their content. The rights usually lie with the publishers or the external rights holders. [See Legal notice.](#)

Download PDF: 06.10.2024

ETH-Bibliothek Zürich, E-Periodica, <https://www.e-periodica.ch>

Tuned Mass Dampers for Continuous Beams

Amortisseurs de masse pour poutres continues

Abgestimmte Tilger für Durchlaufträger

Eilif SVENSSON

Chief Engineer
ES-Consult
Vedbaek, Denmark

Eilif Svensson, born 1945, obtained his MSc and PhD degree from the Technical University of Denmark. Since he has been working in civil and structural engineering as a consulting and research engineer with particular emphasis on stability and dynamic in relation to larger steel structures.

Carsten Munk PLUM

Chief Engineer
ES-Consult
Vedbaek, Denmark

Carsten Munk Plum, born 1948, obtained his MSc degree from the Technical University of Denmark. He has been working in civil and structural engineering since his graduation as a consulting and research engineer within the field of stability and dynamics in relation to larger steel and concrete structures.

SUMMARY

Structural serviceability of buildings also includes comfort of occupants during structural response to dynamic excitation. Excessive dynamic response compromising comfort and serviceability can be suppressed by means of tuned mass dampers. Attention is focused upon the case, where this technique is used to suppress dynamic vibration of continuous beams often characterized by closely spaced frequencies and modes. A theory is presented in which modal analysis plays the key role. Analytical expressions for the response are presented and applied to an example, thereby highlighting some important observations to be accounted for in the tuning strategy.

RESUME

L'aptitude au service de bâtiments comprend aussi le confort humain dans le bâtiment sous excitation dynamique. L'atténuation des oscillations excessives se fait souvent avec des amortisseurs de masse. Le cas d'une poutre continue, pour laquelle les fréquences souvent sont très serrées, est résolu avec plusieurs amortisseurs de masse. Les auteurs présentent une théorie opérationnelle basée sur une analyse modale. Cette analyse détermine la réponse structurelle et un exemple est présenté pour souligner certaines observations importantes dans le cas d'amortissement optimal.

ZUSAMMENFASSUNG

Die Gebrauchstauglichkeit von Gebäuden umfasst viele Komponenten, von denen sich eine auf den Komfort der Benutzer während der Reaktion des Gebäudes auf eine dynamische Anregung bezieht. Eine wohlbekannt Methode zur Unterdrückung übermäßiger dynamischer Reaktionen besteht in der Anwendung von abgestimmten Tilgern. Hier wird insbesondere auf den Fall von Durchlaufträgern eingegangen, die oft durch eng beieinander liegende Frequenzen und Modalformen gekennzeichnet sind. Es wird eine Theorie dargestellt, bei der die Analyse der Schwingungsmoden eine Schlüsselrolle spielt. Es werden analytische Formeln für die Reaktion vorgestellt; anhand eines Beispiels werden einige wichtige Erfahrungen hervorgehoben, die bei der Strategie für die Abstimmung berücksichtigt werden sollten.



1. INTRODUCTION

Dynamic structural response is an important issue related to serviceability of buildings. The source of dynamic excitation is environmental loads such as wind load on tall buildings or occupant activities, installed equipment and machinery. Development of stronger materials enhance dynamic serviceability problems and remedial actions aiming at not exceeding limit values for human exposure to vibrations [1], [2] often is called for. Many of the efficient remedial actions belong to the categories of active or passive damping. Tuned mass dampers belonging to the second category have been used with success [2], [3].

The concept of tuning [2] is well known for ODOF systems. This paper presents a theoretical and operational basis for optimum tuning strategies for continuous beams for which tuning involves more modes. Modal analysis is applied. Operating in the complex plane, explicit determination of the modal response of the damped system as well as the response of the dampers are presented. Through examples application of the theory is elucidated.

2. MODAL ANALYSIS OF CONTINUOUS BEAMS

The well known modal theory for continuous beams is briefly outlined in the following. Using well known symbols the free vibration problem for beams is governed by

$$EI \frac{\partial^4 W}{\partial x^4} + m \frac{\partial^2 W}{\partial t^2} = 0 \quad (1)$$

Modes $F(x)$ and corresponding cyclic eigenfrequencies ω are determined by (2) in which non-dimensional quantities have been introduced

$$A[F] - \lambda B[F] = 0, \quad A[\] = \frac{d^4[\]}{d\xi^4}, \quad B[\] = [\], \quad \xi = \frac{x}{L}, \quad \lambda = \frac{m\omega^2 L^4}{EI} \quad (2)$$

Operators A and B are selfadjoint and defining $\langle f, g \rangle$ as the integration from 0 to 1 of $f(\xi)g(\xi)$

$$\langle A[F], G \rangle = \langle A[G], F \rangle, \quad \langle B[F], G \rangle = \langle F, G \rangle = \langle G, F \rangle = \langle B[G], F \rangle \quad (3)$$

Therefore modes $F_i(\xi)$, $F_j(\xi)$ corresponding to distinct eigenvalues λ_i and λ_j are orthogonal i.e

$$\langle A[F_i], F_j \rangle = \langle B[F_i], F_j \rangle = \int_0^1 F_i(\xi) F_j(\xi) d\xi = 0 \quad \text{for } i \neq j \quad (4)$$

Furthermore we are free to normalise modes with respect to the operator B, therefore

$$\langle B[F_i], F_j \rangle = \langle F_i, F_j \rangle = \delta_{ij}, \quad \langle A[F_i], F_j \rangle = \lambda_j \delta_{ij} \quad (5)$$

To illustrate the phenomena of closely spaced eigenfrequencies of continuous beams two examples are shown on fig. 1 and 2. A simple and efficient technique has been used to identify the modes - based on Fourier expansion of $F(\xi)$ with special modelling at the supports, [4].

Adding a load term to (1), we obtain the equation for forced vibrations

$$EI \frac{\partial^4 W}{\partial x^4} + m \frac{\partial^2 W}{\partial t^2} = p(x,t) \quad (6)$$

Modal expansion of W in the orthonormalized system of modes $F_j(\xi)$

$$W(\xi,t) = \sum_{j=1} a_j(t) F_j(\xi) \quad (7)$$

combined with (5), (6) determines $a_j(t)$. Assuming harmonic excitation $p_j(\xi,t) = p(\xi)e^{i\omega t}$ and including modal damping ζ_j , the modal components a_j are determined by

$$\frac{d^2 a_j}{dt^2} + 2\zeta_j \omega_j \frac{da_j}{dt} + \omega_j^2 a_j = p_j^0 e^{i\omega t}, \quad p_j^0 = \frac{1}{m} \int_0^1 p(\xi) F_j(\xi) d\xi \quad (8)$$

Introducing the well known frequency response function and phase angle of the j 's mode $\Phi(\Omega_{(j)}, \zeta_j)$ and $\varphi_j(\zeta_j, \Omega_{(j)})$ where $\Omega_{(j)} = \omega/\omega_j$ the response can be expressed :

$$W(\xi,t) = W(\xi) e^{i\omega t}, \quad W(\xi) = \sum_{j=1} \frac{1}{\omega_j^2} p_j^0 \Phi(\Omega_{(j)}) e^{i\varphi_j} F_j(\xi) \quad (9)$$

For ω sufficiently close to ω_j the response might become unacceptably high. Applying modal theory as used above, we extend the theory to include the effect of tuned mass dampers.

3. CONTINUOUS BEAMS WITH TUNED MASS DAMPERS

N_D mass dampers are placed at ξ_j , $j=1, 2, \dots, N_D$. The mass, damping and tuning frequency respectively is M_{Dj} , ξ_{Dj} and ω_{Dj} . The exciting frequency is ω and the response is determined by:

$$\frac{\partial^2 W}{\partial t^2} + \frac{EI}{L^4 m} \frac{\partial^4 W}{\partial \xi^4} = \frac{1}{m} p(\xi) e^{i\omega t} - \sum_{k=1}^{k=N_D} \mu_k \frac{d^2 y_k}{dt^2} \delta(\xi - \xi_k) \quad (10)$$

$$\frac{d^2 y_j}{dt^2} + 2\zeta_{Dj} \omega_{Dj} \left(\frac{dy_j}{dt} - \frac{\partial W}{\partial t} \Big|_{\xi=\xi_j} \right) + \omega_{Dj}^2 (y_j - W_{\xi=\xi_j}) = 0 \quad (11)$$

δ is the Kronecker delta, $\mu_j = M_{Dj}/mL$ the mass ratio and y_j the movement of the j 'th damper. Introducing (7), including N_m terms and using (5), - (10) and (11) take the form :

$$\frac{d^2 a_j}{dt^2} + 2\zeta_j \omega_j \frac{da_j}{dt} + \omega_j^2 a_j = p_j^0 e^{i\omega t} - \sum_{k=1}^{k=N_D} \mu_k \frac{d^2 y_k}{dt^2} F_j(\xi_k) \quad j=1, 2, \dots, N_m \quad (12)$$

$$\frac{d^2 y_j}{dt^2} + 2\zeta_{Dj} \omega_{Dj} \left(\frac{dy_j}{dt} - \sum_{k=1}^{k=N_m} \frac{da_k}{dt} F_k(\xi_j) \right) + \omega_{Dj}^2 \left(y_j - \sum_{k=1}^{k=N_m} a_k F_k(\xi_j) \right) = 0, \quad j=1, 2, \dots, N_D \quad (13)$$



We write the solution in the form (14) and make the variable transformation (15), (16):

$$a_j = a_j^0 e^{i(\omega t + \varphi_j)} \quad , \quad y_j = y_j^0 e^{i(\omega t + \psi_j)} \quad (14)$$

$$j=1 \dots N_m : \quad z_{2j-1} = a_j^0 \cos \varphi_j \quad , \quad z_{2j} = a_j^0 \sin \varphi_j \quad (15)$$

$$j=1 \dots N_D : \quad z_{2N_m+2j-1} = y_j^0 \cos \psi_j \quad , \quad z_{2N_m+2j} = y_j^0 \sin \psi_j \quad (16)$$

(14) to (16) is introduced into (12) and (13) together with the definitions

$$F_{jk} = F_j(\xi_k) \quad , \quad \Omega_{(Dj)} = \frac{\omega}{\omega_{(Dj)}} \quad : \quad j=1 \dots N_m \quad , \quad k=1 \dots N_D \quad (17)$$

whereby $N=2(N_m+N_D)$ equations for the N unknown z_i have been determined :

$1 \leq j \leq N_m$

$$(1 - \Omega_{(j)}^2) z_{2j-1} - 2\zeta_j \Omega_{(j)} z_{2j} - \Omega_{(j)}^2 \sum_{k=1}^{k=N_D} \mu_k F_{jk} z_{2N_m+2k-1} = \frac{p_j^0}{\omega_j^2} \dots eq(2j-1)$$

$$2\zeta_j \Omega_{(j)} z_{2j-1} + (1 - \Omega_{(j)}^2) z_{2j} - \Omega_{(j)}^2 \sum_{k=1}^{k=N_D} \mu_k F_{jk} z_{2N_m+2k} = 0 \dots eq(2j)$$

$1 \leq j \leq N_D$

$$-\sum_{k=1}^{k=N_m} F_{jk} z_{2k-1} + 2\zeta_{Dj} \Omega_{(Dj)} \sum_{k=1}^{k=N_m} F_{jk} z_{2k} + (1 - \Omega_{(Dj)}^2) z_{2N_m+2j-1} - 2\zeta_{Dj} \Omega_{(Dj)} z_{2N_m+2j} = 0 \dots eq(2N_m+2j-1)$$

$$-2\zeta_{Dj} \Omega_{(Dj)} \sum_{k=1}^{k=N_m} F_{kj} z_{2k-1} - \sum_{k=1}^{k=N_m} F_{kj} z_{2k} + 2\zeta_{Dj} \Omega_{(Dj)} z_{2N_m+2j-1} + (1 - \Omega_{(Dj)}^2) z_{2N_m+2j} = 0 \dots eq(2N_m+2j)$$

$$\text{That is :} \quad K_{ij} z_j = b_i \quad (18)$$

after having solved these for z_i (note K_{ij} is not symmetric) we find for instance:

$$a_j^0 = \text{sign}(z_{2j-1}) \sqrt{z_{2j-1}^2 + z_{2j}^2} \quad , \quad \varphi_j = \text{Arctan}\left(\frac{z_{2j}}{z_{2j-1}}\right) \quad : \quad j=1 \dots N_m \quad (19)$$

and for the response (similar expression for the damper displacement) :

$$W(\xi, t) = W(\xi) e^{i\omega t}, \quad W(\xi) = \sum_{j=1}^{j=N_m} a_j^0 e^{i\varphi_j} F_j(\xi) \quad (20)$$

3.1 Application, example

The beam shown in fig. 1 and 3 must be damped for exiting frequencies ω in an interval including the four lowest modes. The analysis includes 12 modes i.e. $N_m = 12$. The number of dampers are 4, hence $N_D = 4$ and N in (18) is $= 2(12 + 4) = 32$. Having determined the 12 modes according to the method briefly outlined in section 1, p_j^0 and the coefficients F_{jk} are determined. ω and the chosen tuning frequencies ω_{Di} determines $\Omega_{(i)}$ and $\Omega_{(Dj)}$. At this point - for each exiting frequency - K_{ij} and correspondingly all information regarding the response can be extracted from (18) to (20).

The tuning strategy must according to our experience observe the following rules: 1) Mass damper j , aiming at suppressing resonant response of mode j , should preferably be placed where mode no. j has it maximum value. 2) In addition to obeying rule no 1, care should be taken not to place dampers damping modes ω_i and ω_{i+1} close to each other as the dampers then will be working in opposite phase and cancel each other in the frequency range ω_i to ω_{i+1} . For instance if damper D2 is moved close to damper D1 and D4 is moved to a position near D2 we still obey rule no 1, but the damping arrangement would be very inefficient in the frequency range $\omega = 1.00$ to 1.15 .

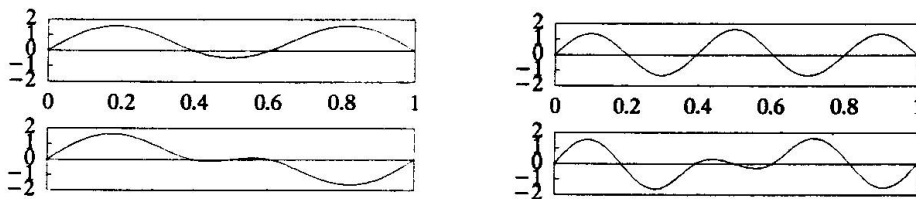


FIG. 1 Continuous beam over two internal supports. Length of span 1 and 3: $0.39474 L$ and of span 2 $0.21052 L$, where L is the total length. Shown from top to bottom - from left to right orthonormalized modes no. 1 to 4, which are lumped in pairs: $\sqrt{\lambda_i} = 76.8, 87.1$ and $246, 288$ for $i=1$ to 4. $\lambda_i = m\omega_i^2 L^4/EI$.

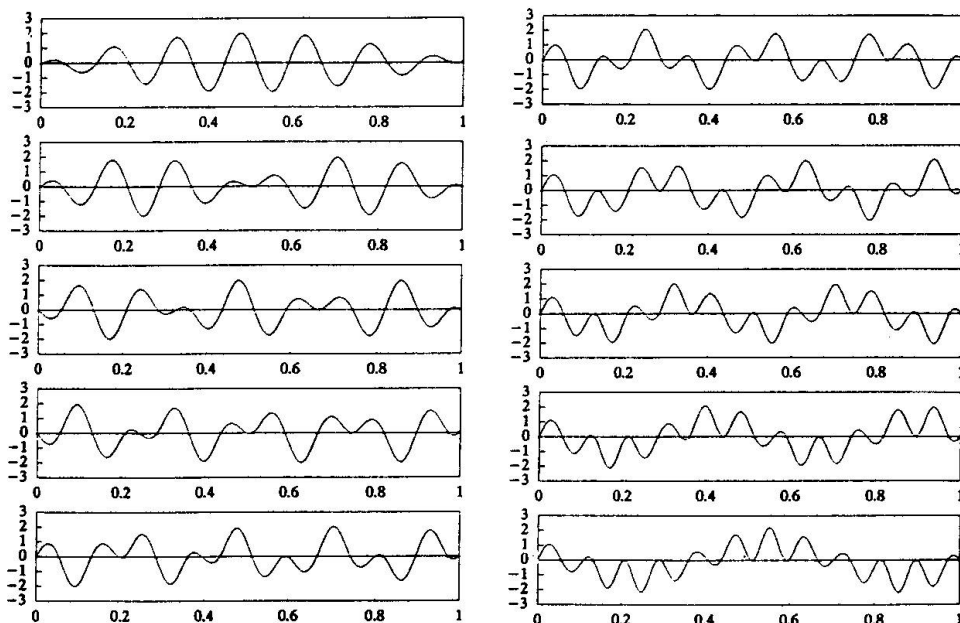


FIG. 2 Continuous beam over 13 internal supports. Length of span 1: $0.05536 L$, length of span 2 to 13: $0.07631 L$ length of span 14: $0.028865 L$. Shown from top to bottom from left to right orthonormalized modes 1 to 10. Eigenfrequencies only differ by a factor 2 from mode 1 to mode 10: $\sqrt{\lambda_i} = 1723, 1805, 1934, 2103, 2302, 2524, 2762, 3010, 3257$ and 3492 for $i=1$ to 10. $\lambda_i = m\omega_i^2 L^4/EI$

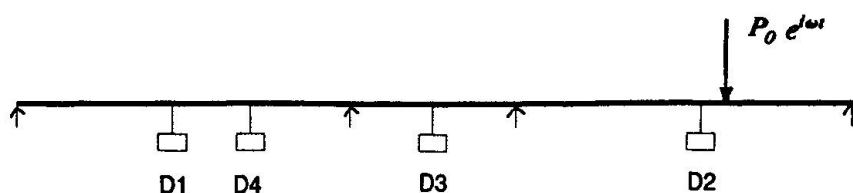


FIG. 3 Beam of fig.1. Single force acts at $\xi=0.85$ at exciting frequencies $0 \leq \omega \leq 4$. Four tuned mass dampers placed at $\xi_1=0.185$, $\xi_2=0.82$, $\xi_3=0.50$, $\xi_4=0.28$. 12 modes have been included in the analysis: ω_i , $i=1$ to 12: 1.000, 1.135, 3.202, 3.750, 5.016, 7.858, 8.397, 12.75, 14.08, 15.99, 21.28 and 22.16. Modal damping $\zeta_i=0.01$. The following tuning frequencies, mass ratio and damping values for the tuned mass dampers have been applied: $\omega_{Dj}=0.99$, 1.124, 3.17 and 3.713. $\mu_{Dj}=2\%$ and for ζ_{Dj} ($j=1$ to 4) two set of values have been used: 0.15 and 0.30.

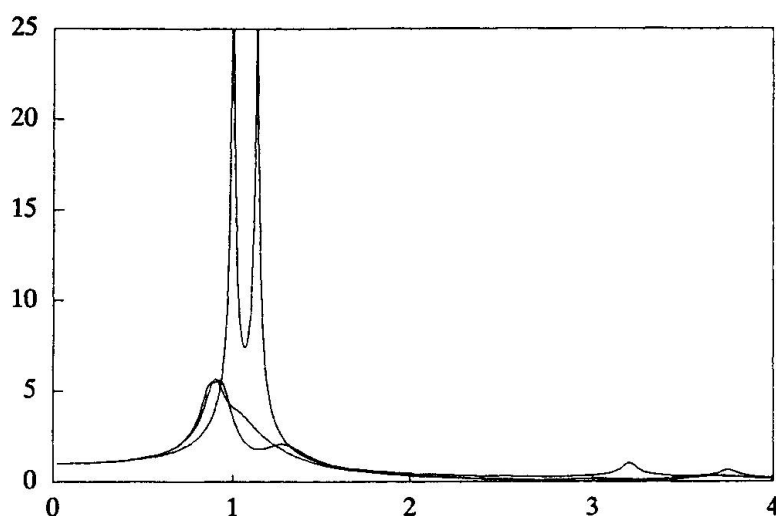


FIG. 4 Frequency response for the continuous beam of fig 1 and 3. x- axis frequency - y axis: the amplitude of the beam at the point where the load is applied. The two lower curves show the response for the damper configuration of fig 3 - data as given in legend to fig 2. The dampers reduce the peak values of the undamped system by a factor 5 to 10.

4. CONCLUDING REMARKS

A theory applicable to the case of dynamical excited beams being damped with tuned mass dampers has been presented. Based on the theory a user friendly edp based numerical tool enabling the designer to implement the optimum damping arrangement will be developed. The mass ratio to be applied to obtain efficient damping is very small (2% or less) - therefore the mechanical devices acting as tuned mass dampers can be very simple. Another potential avenue for future work of course will be to propose a simple design for such devices.

REFERENCES

1. Bachmann, H and W. Ammann, *Vibrations in Structures*, IABSE Struct. Engr. Doc. 3e, 1987
2. ISO 2631/1-3, *Evaluation of human exposure to whole-body vibration*, 1985 and 1989
3. Gerasch, W.J. and H.G. Natke, *Vibration reduction of two Structures*, Int. Symp. on Vibration Protection in Construction, Leningrad 1984
4. Svensson, E., Non-published notes, 1992